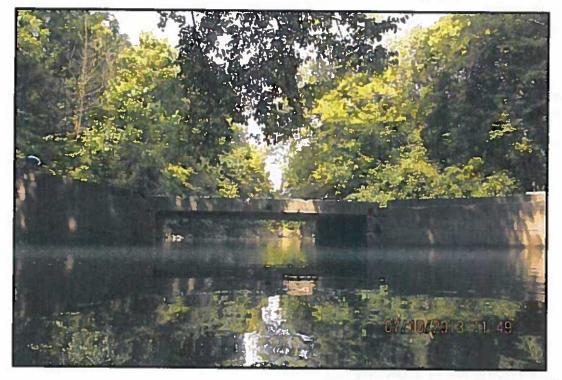


UNDERWATER BRIDGE INSPECTION REPORT STRUCTURE NO. 108B00041N KY-2885 OVER SIMPSON CREEK SPENCER COUNTY, KY

DISTRICT 5



JULY 2013

PREPARED FOR THE





KENTUCKY TRANSPORTATION CABINET

124 Venture Court, Suite 10 • Lexington, KY 40511 • 859.367.0097 • Fax 859.367.0140 • www.collinsengr.com

Structure No. 108B00041N

KY-2885 over Simpson Creek

UNDERWATER INSPECTION SUMMARY

GENERAL:					
Inspection Date:	July 10, 2013				
Inspection Team Leader:	Joshua M. Johnson, P.E.				
Support-Divers:	Adam R. Davis, P.E.; Allen D. Cantrell, E.I.T.				
Inspection Method:	Surface Supplied Air				
BRIDGE INFORMATION:					
Superstructure Type:	Single Simple Span of Precast Prestressed Concrete Box Beam				
Substructure Type:	Reinforced Concrete Abutments				
SSUs in Water:	Abutments 1 and 2				
Year Constructed:	1981				
GENERAL CONDITIONS:					
Water Visibility:	2 feet	Water Velocity:	< 1 ft/s		
Water Temperature:	65 °F	Weather:	Overcast - 90 °F		
Waterline Elevation:	97.0 feet	Approx. HW Mark:	N/A		
Waterline Reference:	Bottom of the D	eck at the Downstream End of	Abutment I		
Maximum Depth at SSU:	9.3 feet – Downstream End of Abutment 2				
Shoreline Conditions:	Moderately Vegetated Embankments with Minor Erosion				

SUBSTRUCTURE CONDITIONS:

Abutment 1: Concrete surfaces exhibited light scaling from the waterline to the channel bottom. Vertical cracking and spalling was noted in areas of previous repair. See Figure 4 in Appendix A for detailed inspection notes.

Abutment 2: Concrete surfaces exhibited light scaling from the waterline to the channel bottom. The footing along the south face was undermined and shear cracks were noted on both wingwalls. Construction joints were separated on both wingwalls. Spalling was noted in areas of previous repair. See Figure 5 in Appendix A for detailed inspection notes.

RECOMMENDATIONS:

Replace structure.

Perform load rating analysis and impose load restrictions on this structure based on results. Perform structural repairs to Abutment 2 until a replacement bridge can be constructed.

UNDERWATER INSPECTION CODING:

NBI Ratings:

Item	Description	Coding	Condition
60	Substructure	2 – Critical Condition	Advanced Deterioration & Undermining
61	Channel	7 – Good Condition	Minor Erosion
92B	UW Insp. Frequency	12 Months	Inspector Recommended
93B	Insp. Date	07 10 13	
113	Scour Critical Bridges	3 – Scour Critical	Unstable (Inspector Recommended)

AASHTO Element (CoRe) Ratings:

				Condition State			
Element #	Description	Units	Total	1	2	3	4
215	Abutment	LF	38	0	0	19	19

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.

Joshua M. Johnson, P.E

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2013 Underwater Bridge Inspection Report

1.0 INTRODUCTION

1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the KY-2885 Bridge over Simpson Creek in Spencer County, KY. Collins Engineers, Inc. (Collins) conducted the underwater investigation for the Kentucky Transportation Cabinet (KYTC) on July 10, 2013. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

1.2 General Description of the Structure

Structure No. 108B00041N spans 22 feet, carrying KY-2885 over Simpson Creek and is approximately 18 feet wide. The bridge superstructure is constructed of one precast prestressed concrete box beam span. The roadway orientation of the longitudinal axis of the bridge is south to north. The substructure units are labeled as Abutments 1 and 2. Existing design drawings were not available at the time of the inspection. Refer to Figure 1 in Appendix A for a Location Map of the bridge. Refer to Photographs 1 and 2 Appendix B for overall views of the bridge.

1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.

A three-person team consisting of two professional engineer-divers and team leader (Joshua M. Johnson, P.E. and Adam R. Davis, P.E.) and an engineer-diver (Allen D. Cantrell, E.I.T.) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the divers worked from the shore and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the substrate condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

The channel bottom was sounded utilizing a digital fathometer and telescoping survey rod. Soundings were taken along both fascias and approximately 50 feet upstream and downstream of the bridge. Soundings were also taken along the pier faces and noses, between the piers along the structure centerline, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and mapping of the shoreline. Refer to Figures 2 and 3 in Appendix A for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

2.0 EXISTING CONDITIONS

2.1 General Conditions

At the time of the inspection, the waterline of Simpson Creek was located approximately 3.0 feet below the bottom of deck at the west face of Abutment 1, which corresponds to an assumed waterline elevation of 97.0 feet. During the inspection the waterway was flowing less than 1 foot per second. There were indications of localized scour on the south face of Abutment 2. The shorelines adjacent to the bridge consisted of medium dense clay and riprap and exhibited minor erosion. Refer to Figures 2 through 5 in Appendix A for detailed inspection notes on the substructure units. Refer to Photographs 3 through 6 in Appendix B for views of the shorelines

KY-2885 over Simpson Creek

Based on the critical findings at Abutment 2, it is recommended that the submerged substructure units of Structure No. 108B00041N be next inspected underwater at an interval not to exceed 12 months. Topside inspections are currently being conducted by the District after each major rain event.

Respectfully submitted,

COLLINS ENGINEERS, INC.

Joshua M. Johnson, P.E. Field Team Leader

Originated By: Allen D. Cantrell, E.I.T.

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near the structure. The channel bottom material around the abutments consisted of bedrock with no probe rod penetration.

2.2 Substructure Conditions

Abutment 1

The concrete below the water surface on Abutment 1 exhibited light scaling with up to 1/8 inch of section loss. The footing is socketed into bedrock and no undermining was observed. Vertical cracking up to 1/16 inch wide from 18 inches below the waterline to the bottom of the deck was noted in areas of previous repair. Spalling up to 2 feet by 2 feet with 8 inches of penetration and exposed reinforcement was also observed in areas of previous repair. Refer to Photographs 7 through 10 in Appendix B for views of Abutment 1.

Abutment 2

The concrete below the water surface on Abutment 2 exhibited light scaling with up to 1/8 inch of section loss. The footing is socketed into bedrock, however the rock socket is oversized and the backfill material is eroding during high flow events. Undermining was observed on the south face with a vertical exposure of 2 feet and up to 3 feet of penetration. The backfill material consists of stiff clay with 6 inches of penetration. Shear cracking up to 1/2 inch wide from bottom of footing to the bottom of the deck was noted on both wingwalls. Construction joints on both wingwalls were separating with observed dimensions of up to 1/2 inch wide. Spalling up to 6 feet by 3 feet with 3 inches of penetration and exposed reinforcement was also observed in areas of previous repair. Refer to Photographs 11 through 18 in Appendix B for views of Abutment 2.

3.0 EVALUATION AND RECOMMENDATIONS

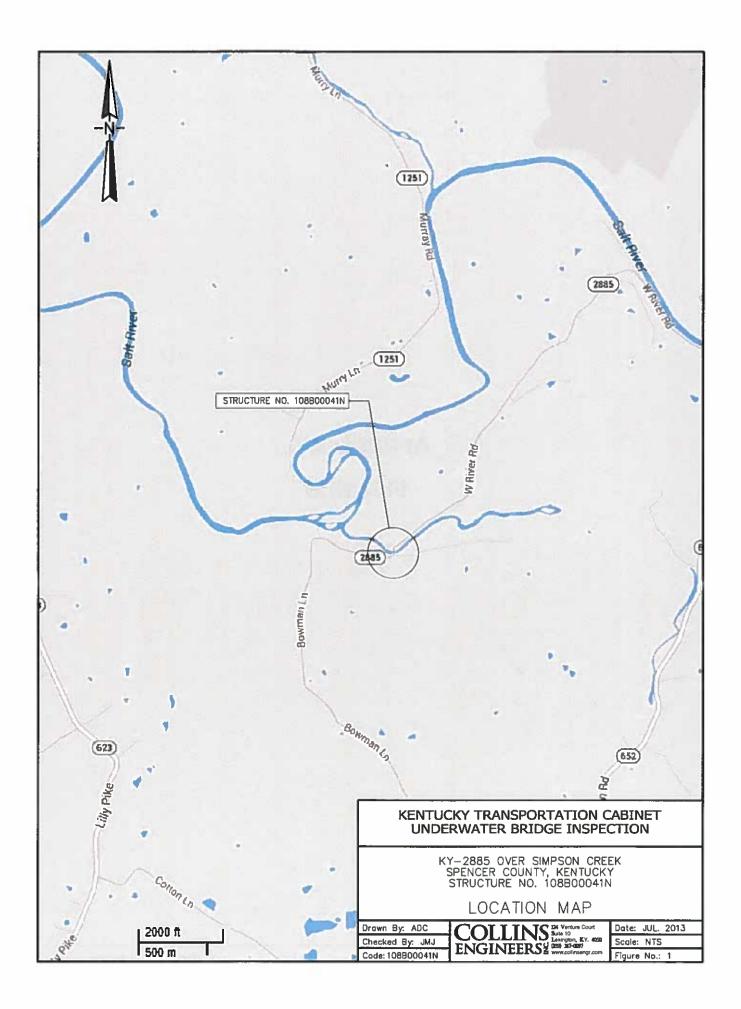
The inspected substructure units of Structure No. 108B00041N were in critical condition due to contraction scour at Abutment 2. A load rating should be performed and load restrictions should be placed on this structure based on those results. This bridge should be replaced on a priority basis. Structural repairs to Abutment 2 should be implemented until a replacement bridge can be constructed.

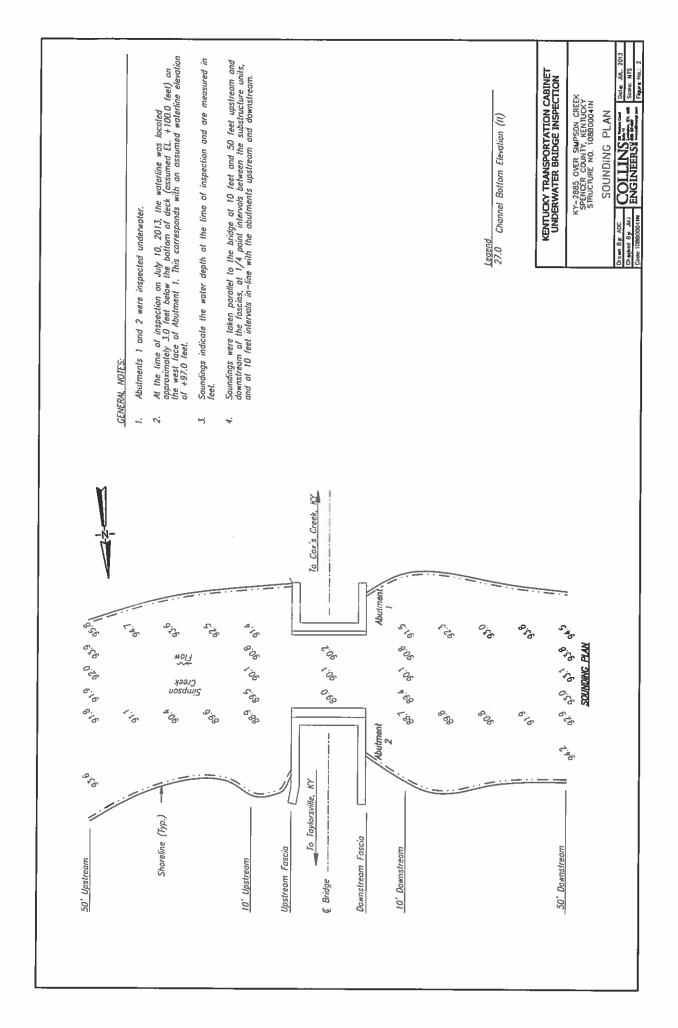
APPENDIX A: FIGURES

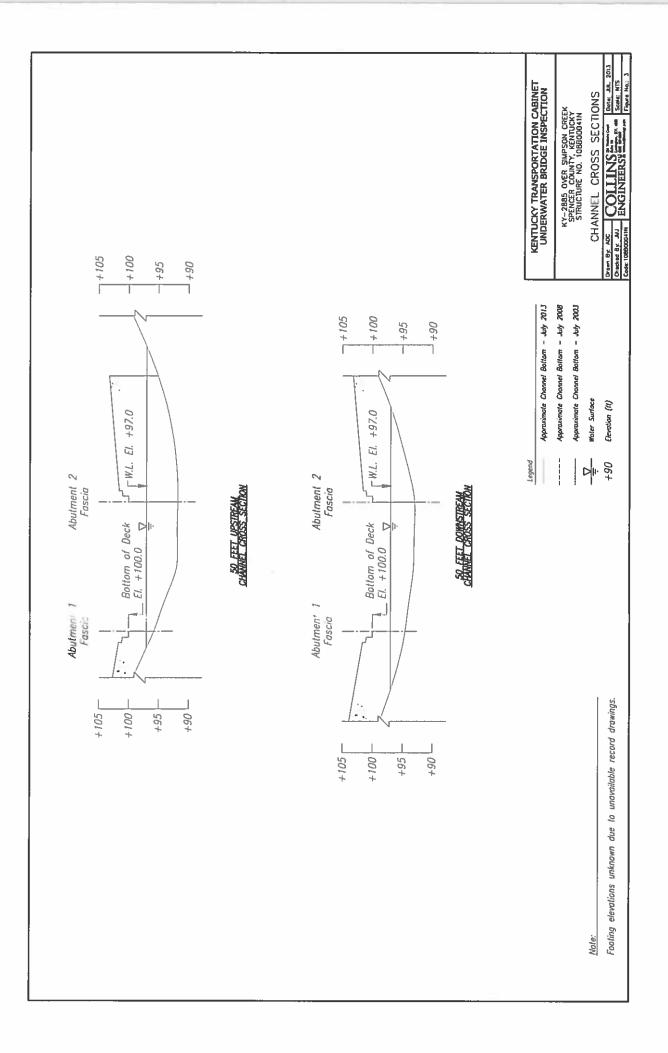
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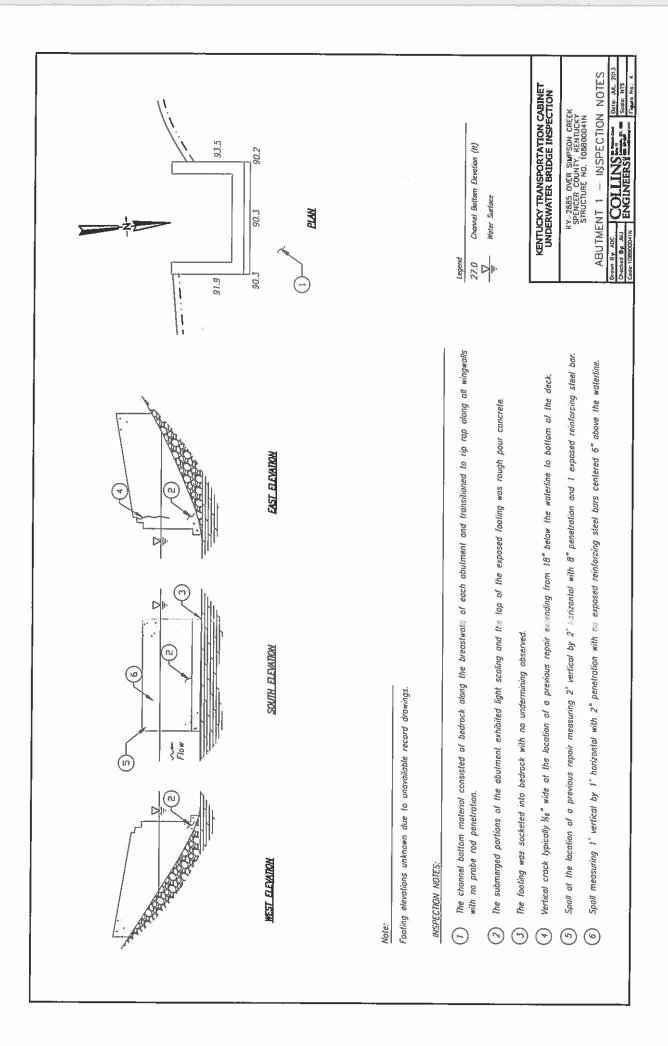
Collins Engineers, Inc.

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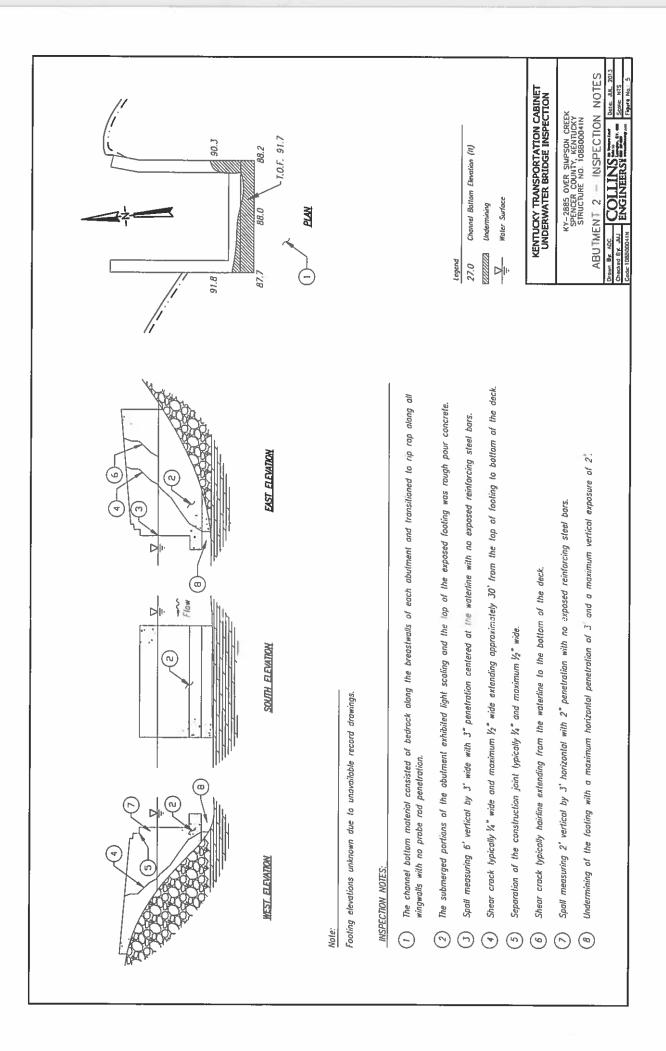


APPENDIX B: PHOTOGRAPHS

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Photograph No. 1: Overall View of Structure No. 108B00041N, Looking West.



Photograph No. 2: Overall View of Structure No. 108B00041N, Looking East.

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Photograph No. 3: View of North Embankment Upstream of the Structure, Looking Northeast.



4: View of North Embankment Downstream of the Structure, Looking Northwest.

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View of South Embankment Downstream of the Structure, Looking Southwest.

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Photograph No. 7: View of East Wingwall of Abutment 1, Looking Southwest.



Photograph No. 8: View of Abutment 1, Looking Southwest.

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Photograph No. 9: View of West Wingwall of Abutment 1, Looking Southeast.



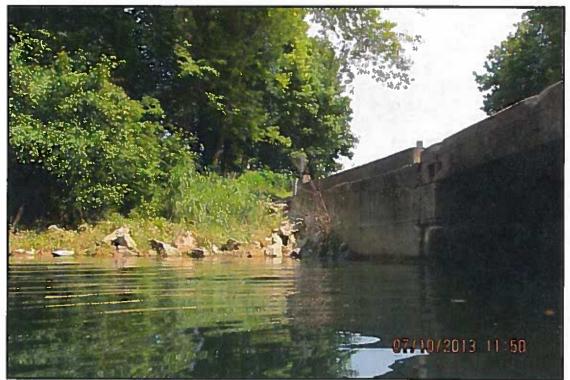
Photograph No. 10: View of Failed Concrete Repair at Upstream Corner of Abutment 1, Looking South.

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Photograph No. 13: View of West Wingwall of Abutment 2, Looking Northeast.



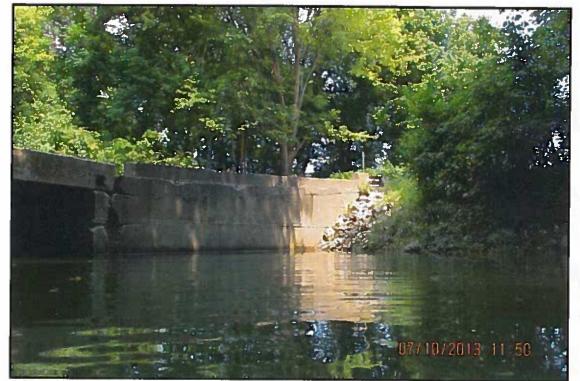
Photograph No. 14: View of Shear Crack above the Water Line on East Wingwall of Abutment 2, Looking West.

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Photograph No. 11: View of East Wingwall of Abutment 2, Looking Northwest.

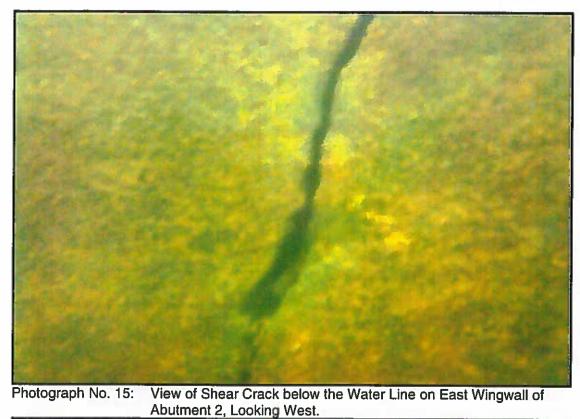


Photograph No. 12: View of Abutment 2, Looking Northwest.



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Photograph No. 16: View of Spall on East Wingwall of Abutment 2, Looking Southwest.

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Photograph No. 17: View of Shear Crack above the Water Line on West Wingwall of Abutment 2, Looking East.



Photograph No. 18: View of Spall on West Wingwall of Abutment 2, Looking Southeast.

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